We are IntechOpen, the world's leading publisher of Open Access books Built by scientists, for scientists



186,000

200M



Our authors are among the

TOP 1% most cited scientists





WEB OF SCIENCE

Selection of our books indexed in the Book Citation Index in Web of Science™ Core Collection (BKCI)

Interested in publishing with us? Contact book.department@intechopen.com

Numbers displayed above are based on latest data collected. For more information visit www.intechopen.com



Stochastic Finite Element Method in Mechanical Vibration

Mo Wenhui Hubei University Of Automotive Technology China

1. Introduction

Material properties, geometry parameters and applied loads of the structure are assumed to be stochastic. Although the finite element method analysis of complicated structures has become a generally widespread and accepted numerical method in the world, regarding the given factors as constants can not apparently correspond to the reality of a structure.

The direct Monte Carlo simulation of the stochastic finite element method(DSFEM) requires a large number of samples, which requires much calculation time and occupies much computer storage space [1]. Monte Carlo simulation by applying the Neumann expansion (NSFEM) enhances computational efficiency and saves storage in such a way that the NSFEM combined with Monte Carlo simulation enhances the finite element model advantageously [2]. The preconditioned Conjugate Gradient method (PCG) applied in the calculation of stochastic finite elements can also enhance computational accuracy and efficiency [3]. The TSFEM assumes that random variables are dealt with by Taylor expansion around mean values and is obtained by appropriate mathematical treatment [4, 14]. According to first-order or second-order perturbation methods, calculation formulas can be obtained [2, 5, 6,8, 9, 13, 15, 16]. The result is called the PSFEM and has been adopted by many scholars.

The PSFEM is often applied in dynamic analysis of structures and the second- order perturbation technique has been proved to be accurate and efficient. Dynamic reliability of a large frame is calculated by the SFEM and response sensitivity is formulated in the context of stiffness and mass matrix condensation [7]. Nonlinear structural dynamics are developed by the PSFEM. Nonlinearities due to material and geometrical effects have also been included [8]. By forming a new dynamic shape function matrix, dynamic analysis of the spatial frame structure is presented by the PSFEM [9].

It is significant to extend this research to the dynamic state. Considering the influence of random factors, the mechanical vibrations for a linear system are illustrated by using the TSFEM and the CG.

2. Random variable

Material properties, geometry parameters and applied loads of machines are assumed to be independent random variables, and are indicated as $a_1, a_2, \dots, a_i, \dots, a_{n_1}$. Their means are $\mu_1, \mu_2, \dots, \mu_i, \dots, \mu_{n_1}$, and their variances are... $\sigma_i^2, \dots, \sigma_{n_1}^2$. When they are subject to

normal distributions, the standard method used to simulate them is to take advantage of well-tested computer programs. When they are subject to unknown distributions, the sample of $a_1, a_2, \dots, a_i, \dots, a_{n_1}$ can be generated from the following method:

$$P\{|x-\mu'| \ge \varepsilon\} \le \frac{\sigma^2}{\varepsilon^2} \tag{1}$$

where, *x* is a random variable, μ' is the mean, σ is the standard deviation ,and ε is an arbitrary positive number. Eq.1 is called the Chebyschev inequality.

The Chebyschev inequality can also be written as

$$P\{|x-\mu'|<\varepsilon\} \ge 1 - \frac{\sigma^2}{\varepsilon^2}$$
⁽²⁾

After substituting $\varepsilon = 6\sigma_i$, $x = a_i$, Eq.2 becomes

$$P\{|a_i - \mu_i| < 6\sigma_i\} \ge 0.9722 \tag{3}$$

where

$$a_i < 6\sigma_i + \mu_i \tag{4}$$

or

$$a_i > -6\sigma_i + \mu_i \tag{5}$$

If it is assumed that z' is a random number within the open interval (0,1), then

$$a_i = 6\sigma_i z' + \mu_i \tag{6}$$

or

$$a_i = -6\sigma_i z' + \mu_i \tag{7}$$

Large numbers of samples of random variables $a_1, a_2, \dots, a_i, \dots, a_{n_1}$ are produced from Eqs.6 and 7 so as to resolve the stochastic finite element problem through Monte Carlo stimulation.

3. Dynamic analysis of finite element

For a linear system, the dynamic equilibrium equation is given by

$$[M]\{\dot{\delta}\} + [C]\{\dot{\delta}\} + [K]\{\delta\} = \{F\}$$
(8)

where $\{\vec{\delta}\}, \{\vec{\delta}\}, \{\vec{\delta}\}\$ are the acceleration, velocity and displacement vectors. [M], [K] and [C] are the global mass, stiffness and damping matrices obtained by assembling the element variables in global coordinate system.

In order to program easily, the comprehensive calculation steps of the Newmark method are as follows

The matrices [K], [M] and [C] are formed.

The initial values $\{\delta_t\}, \{\dot{\delta}_t\}, \{\ddot{\delta}_t\}$ are given.

After selecting step Δt and parameters γ , β , the following relevant parameters are calculated:

$$\gamma \ge 0.50 \qquad \beta \ge 0.25 (0.5 + \gamma)^2$$

$$b_0 = \frac{1}{\beta (\Delta t)^2} \qquad b_1 = \frac{\gamma}{\beta \Delta t} \qquad b_2 = \frac{1}{\beta \Delta t}$$

$$b_3 = \frac{1}{2\beta} - 1 \qquad b_4 = \frac{\gamma}{\beta} - 1 \qquad b_5 = \frac{\Delta t}{2} \left(\frac{\gamma}{\beta} - 2\right)$$

$$b_6 = \Delta t (1 - \gamma) \qquad b_7 = \gamma \Delta t$$

The stiffness matrix is defined as

$$\begin{bmatrix} \tilde{K} \end{bmatrix} = \begin{bmatrix} K \end{bmatrix} + b_0 \begin{bmatrix} M \end{bmatrix} + b_1 \begin{bmatrix} C \end{bmatrix}$$
(9)

The stiffness matrix inversion $\left\lceil \tilde{K} \right\rceil^{-1}$ is solved.

2. Calculation of each step time

At time $t + \Delta t$, the load vector is defined as

$$\left\{\tilde{F}_{t+\Delta t}\right\} = \left\{F_{t+\Delta t}\right\} + \left[M\right] \left(b_0 \left\{\delta_t\right\} + b_2 \left\{\dot{\delta}_t\right\} + b_3 \left\{\ddot{\delta}_t\right\}\right) + \left[C\right] \left(b_1 \left\{\delta_t\right\} + b_4 \left\{\dot{\delta}_t\right\} + b_5 \left\{\ddot{\delta}_t\right\}\right)$$
(10)

At time $t + \Delta t$, the displacement vector is given by

$$\{\delta_{t+\Delta t}\} = \left[\tilde{K}\right]^{-1} \left\{\tilde{F}_{t+\Delta t}\right\}$$
(11)

At time $t + \Delta t$, the velocity vector and acceleration vector are obtained as

$$\left\{\ddot{\delta}_{t+\Delta t}\right\} = b_0\left(\left\{\delta_{t+\Delta t}\right\} - \left\{\delta_t\right\}\right) - b_2\left\{\dot{\delta}_t\right\} - b_3\left\{\ddot{\delta}_t\right\}$$
(12)

$$\left\{\dot{\delta}_{t+\Delta t}\right\} = \left\{\dot{\delta}_{t}\right\} + b_{6}\left\{\ddot{\delta}_{t}\right\} + b_{7}\left\{\ddot{\delta}_{t+\Delta t}\right\}$$
(13)

Vectors $\{\delta_{t+i_1\Delta t}\},\{\dot{\delta}_{t+i_1\Delta t}\},\{\ddot{\delta}_{t+i_1\Delta t}\}\$ are solved at time $t+i_1\Delta t(i_1=2,3,\cdots,n_3)$ step-by -step.

4. Analysis of mechanical vibration based on CG

Eq.11 can be expressed as

$$\begin{bmatrix} \tilde{K} \end{bmatrix} \{ \delta_{t+\Delta t} \} = \{ \tilde{F}_{t+\Delta t} \}$$
(14)

 N_1 samples of random variables $a_1, a_2, \dots, a_i, \dots, a_{n_1}$ are produced. N_1 matrices $\begin{bmatrix} \tilde{K} \end{bmatrix}$ and N_1 Eqs.14 are also generated. For a linear vibration, Eq.14 is the system of linear equations. The CG method is an effective method for solving the large system of linear equations according to the following steps:

1. First, select an approximate solution as the initial value

$$\delta^{(0)}_{t+\Delta t} = \left(\delta^{(0)}_{(t+\Delta t)_{1}}, \delta^{(0)}_{(t+\Delta t)_{2}}, \cdots, \delta^{(0)}_{(t+\Delta t)_{N'}}\right)^{T}$$
(15)
2. Calculate the first residual vector
$$r^{(0)} = \left\{\tilde{F}_{t+\Delta t}\right\} - \left[\tilde{K}\right]\delta^{(0)}_{t+\Delta t}$$
(16)

and vector

$$p^{(0)} = \left[\tilde{K}\right]^T r^{(0)} \tag{17}$$

where, $\begin{bmatrix} \tilde{K} \end{bmatrix}^T$ is the transposed matrix of $\begin{bmatrix} \tilde{K} \end{bmatrix}$ 3. For $\tilde{i} = 0, 1, 2, \dots, n_2 - 1$, iterate step-by-step as follows

$$\alpha_{\tilde{i}} = \frac{\left(\left[\tilde{K}\right]p^{(\tilde{i})}, r^{(\tilde{i})}\right)}{\left(\left[\tilde{K}\right]p^{(\tilde{i})}, \left[\tilde{K}\right]p^{(\tilde{i})}\right)} = \frac{\left(p^{(\tilde{i})}, \left[\tilde{K}\right]^{\mathrm{T}}r^{(\tilde{i})}\right)}{\left(\left[\tilde{K}\right]p^{(\tilde{i})}, \left[\tilde{K}\right]p^{(\tilde{i})}\right)} = \frac{\left(\left[\tilde{K}\right]^{\mathrm{T}}r^{(\tilde{i})}, \left[\tilde{K}\right]^{\mathrm{T}}r^{(\tilde{i})}\right)}{\left(\left[\tilde{K}\right]p^{(\tilde{i})}, \left[\tilde{K}\right]p^{(\tilde{i})}\right)}$$
(18)

$$\{\delta_{t+\Delta t}\}^{(\tilde{i}+1)} = \{\delta_{t+\Delta t}\}^{(\tilde{i})} + \alpha_{\tilde{i}}p^{(\tilde{i})}$$
⁽¹⁹⁾

$$r^{\left(\tilde{i}+1\right)} = r^{\left(\tilde{i}\right)} - \alpha_{\tilde{i}} \left[\tilde{K}\right] p^{\left(\tilde{i}\right)}$$
⁽²⁰⁾

$$\beta_{\tilde{i}+1} = \frac{\left(\left[\tilde{K}\right]^{T} r^{\left(\tilde{i}+1\right)}, \left[\tilde{K}\right]^{T} r^{\left(\tilde{i}+1\right)}\right)}{\left(\left[\tilde{K}\right]^{T} r^{\left(\tilde{i}\right)}, \left[\tilde{K}\right]^{T} r^{\left(\tilde{i}\right)}\right)}$$

$$p^{\left(\tilde{i}+1\right)} = \left[\tilde{K}\right]^{T} r^{\left(\tilde{i}+1\right)} + \beta_{\tilde{i}+1} p^{\left(\tilde{i}\right)}$$
(21)
(22)

The process can be stopped only if r^{n_2} is small enough. Vectors $\{\delta_{t+\Delta t}\}_1, \{\delta_{t+\Delta t}\}_2, \dots, \{\delta_{t+\Delta t}\}_{N_1}$ are solutions of N_1 Eqs.14. The mean of $\{\delta_{t+\Delta t}\}$ is given by

$$\mu\{\delta_{t+\Delta t}\} = \frac{\{\delta_{t+\Delta t}\}_1 + \{\delta_{t+\Delta t}\}_2 + \dots + \{\delta_{t+\Delta t}\}_{N_1}}{N_1}$$
(23)

The variance of $\{\delta_{t+\Delta t}\}$ is given by

$$Var\{\delta_{t+\Delta t}\} = \frac{1}{N_1 - 1} \sum_{i=1}^{N_1} \left(\{\delta_{t+\Delta t}\}_i - \mu\{\delta_{t+\Delta t}\}\right)^2$$
(24)

Similarly, the mean and variance of the vector $\{\delta_{t+i_1\Delta t}\}$ can be solved for at time $t + i_1\Delta t$ ($i_1 = 2, 3, \dots, n_3$) step-by-step.

At time $t' = t + i_2 \Delta t (i_2 = 1, 2, \dots, n_4)$, the strain and stress vectors for element *d* are

$$\{\varepsilon\} = [B]\{\delta_{t'}^{d}\}$$
(25)

and

$$\{\sigma\} = [D]\{\varepsilon\} \tag{26}$$

where, [D] = the material response matrix of element d, [B] = the gradient matrix of element d and $\{\delta_{t'}^{d}\}$ = the element d nodal displacement vector at time t'. Substituting Eq.25 into Eq.26, the stress for element d is given by

$$\{\sigma\} = [D][B]\{\delta_{t'}^{d}\}$$
⁽²⁷⁾

Substituting..samples of random variables $a_1, a_2, \dots, a_i, \dots, a_{n_1}$ into Eq.27, the vectors $\{\sigma\}_1, \{\sigma\}_2, \dots, \{\sigma\}_{N_1}$ can be obtained. The mean of $\{\sigma\}$ is given by

$$\mu\{\sigma\} = \frac{\{\sigma\}_1 + \{\sigma\}_2 + \dots + \{\sigma\}_{N_1}}{N_1}$$
(28)

The variance of $\{\sigma\}$ is given by

$$Var\{\sigma\} = \frac{1}{N_1 - 1} \sum_{i=1}^{N_1} \left(\{\sigma\}_i - \mu\{\sigma\}\right)^2$$
(29)

The CG method belongs to method of iteration with the advantage of quick convergence. For practical purpose, PCG is applied to accelerate the convergence.

5. Analysis of mechanical vibration based on TSFEM

Independent random variables of the system are regarded as $a_1, a_2, \dots, a_i, \dots, a_{n_1}$. The partial derivative of Eq.14 with respect to a_i is given by

$$\frac{\partial\{\delta_{t+\Delta t}\}}{\partial a_{i}} = \left[\tilde{K}\right]^{-1} \left(\frac{\partial\{\tilde{F}_{t+\Delta t}\}}{\partial a_{i}} - \frac{\partial\left[\tilde{K}\right]}{\partial a_{i}}\{\delta_{t+\Delta t}\}\right)$$
(30)

where

$$\frac{\partial \{\tilde{F}_{t+\Delta t}\}}{\partial a_{i}} = \frac{\partial \{F_{t+\Delta t}\}}{\partial a_{i}} + [M] \left(b_{0} \frac{\partial \{\delta_{t}\}}{\partial a_{i}} + b_{2} \frac{\partial \{\tilde{\delta}_{t}\}}{\partial a_{i}} + b_{3} \frac{\partial \{\tilde{\delta}_{t}\}}{\partial a_{i}} \right) + \frac{\partial [M]}{\partial a_{i}} \left(b_{0} \{\delta_{t}\} + b_{2} \{\tilde{\delta}_{t}\} + b_{3} \{\tilde{\delta}_{t}\} \right) + [C] \left(b_{1} \frac{\partial \{\delta_{t}\}}{\partial a_{i}} + b_{4} \frac{\partial \{\tilde{\delta}_{t}\}}{\partial a_{i}} + b_{5} \frac{\partial \{\tilde{\delta}_{t}\}}{\partial a_{i}} \right) + \frac{\partial [C]}{\partial a_{i}} \left(b_{1} \{\delta_{t}\} + b_{4} \{\tilde{\delta}_{t}\} + b_{5} \{\tilde{\delta}_{t}\} \right)$$

$$(31)$$

After $\frac{\partial \{\delta_t\}}{\partial a_i}\Big|_{t=0} = q_0, \frac{\partial \{\dot{\delta}_t\}}{\partial a_i}\Big|_{t=0} = \dot{q}_0, \frac{\partial \{\ddot{\delta}_t\}}{\partial a_i}\Big|_{t=0} = \ddot{q}_0$ are given, Eq.31 can be calculated.

The partial derivative of Eq.30 with respect to a_i is given by

$$\frac{\partial^2 \left\{ \delta_{t+\Delta t} \right\}}{\partial a_i^2} = \left[\tilde{K} \right]^{-1} \left(\frac{\partial^2 \left\{ \tilde{F}_{t+\Delta t} \right\}}{\partial a_i^2} - 2 \frac{\partial \left[\tilde{K} \right]}{\partial a_i} \frac{\partial \left\{ \delta_{t+\Delta t} \right\}}{\partial a_i} - \frac{\partial^2 \left[\tilde{K} \right]}{\partial a_i^2} \left\{ \delta_{t+\Delta t} \right\} \right)$$
(32)

where

$$\frac{\partial^{2}\left\{\tilde{F}_{t+\Delta t}\right\}}{\partial a_{i}^{2}} = \frac{\partial^{2}\left\{F_{t+\Delta t}\right\}}{\partial a_{i}^{2}} + \frac{\partial^{2}\left[M\right]}{\partial a_{i}^{2}}\left(b_{0}\left\{\delta_{t}\right\} + b_{2}\left\{\dot{\delta}_{t}\right\} + b_{3}\left\{\ddot{\delta}_{t}\right\}\right) + 2\frac{\partial\left[M\right]}{\partial a_{i}^{2}}\left(b_{0}\left\{\dot{\delta}_{t}\right\} + b_{3}\left\{\ddot{\delta}_{t}\right\}\right) + \left[M\right]\left(b_{0}\frac{\partial^{2}\left\{\delta_{t}\right\}}{\partial a_{i}^{2}} + b_{2}\frac{\partial^{2}\left\{\dot{\delta}_{t}\right\}}{\partial a_{i}^{2}} + b_{3}\frac{\partial^{2}\left\{\ddot{\delta}_{t}\right\}}{\partial a_{i}^{2}}\right) + \left[M\right]\left(b_{0}\frac{\partial^{2}\left\{\delta_{t}\right\}}{\partial a_{i}^{2}} + b_{2}\frac{\partial^{2}\left\{\dot{\delta}_{t}\right\}}{\partial a_{i}^{2}} + b_{3}\frac{\partial^{2}\left\{\ddot{\delta}_{t}\right\}}{\partial a_{i}^{2}}\right) + \left[C\right]\left(b_{1}\left\{\delta_{t}\right\} + b_{4}\left\{\dot{\delta}_{t}\right\} + b_{5}\left\{\ddot{\delta}_{t}\right\}\right) + 2\frac{\partial^{2}\left[C\right]}{\partial a_{i}^{2}}\left(b_{1}\frac{\partial\left\{\delta_{t}\right\}}{\partial a_{i}} + b_{4}\frac{\partial\left\{\dot{\delta}_{t}\right\}}{\partial a_{i}} + b_{5}\frac{\partial\left\{\ddot{\delta}_{t}\right\}}{\partial a_{i}}\right) + \left[C\right]\left(b_{1}\frac{\partial^{2}\left\{\delta_{t}\right\}}{\partial a_{i}^{2}} + b_{4}\frac{\partial^{2}\left\{\dot{\delta}_{t}\right\}}{\partial a_{i}^{2}} + b_{5}\frac{\partial^{2}\left\{\ddot{\delta}_{t}\right\}}{\partial a_{i}^{2}}\right) \right)$$

$$(33)$$

After $\frac{\partial^2 \{\delta_t\}}{\partial a_i^2}\Big|_{t=0} = r_0, \frac{\partial^2 \{\dot{\delta}_t\}}{\partial a_i^2}\Big|_{t=0} = \dot{r}_0, \frac{\partial^2 \{\ddot{\delta}_t\}}{\partial a_i^2}\Big|_{t=0} = \ddot{r}_0$ are given, Eq.33 can be calculated.

The displacement is expanded at the mean value point $\overline{a} = (\overline{a}_1, \overline{a}_2, \dots, \overline{a}_i, \dots, \overline{a}_{n_1})^T$ by means of a Taylor series. By taking the expectation operator for two sides of above Eq.11, the mean of the displacement is obtained as

$$\mu\{\delta_{t+\Delta t}\} \approx \{\delta_{t+\Delta t}\}\Big|_{a=\overline{a}} + \frac{1}{2} \sum_{i=1}^{n_1} \frac{\partial^2\{\delta_{t+\Delta t}\}}{\partial a_i^2}\Big|_{a=\overline{a}} \cdot \sigma_i^2$$
(34)

where, $\mu \{\delta_{t+\Delta t}\}$ expresses the mean value of $\delta_{t+\Delta t}$. The variance of $\delta_{t+\Delta t}$ is given by

$$Var\{\delta_{t+\Delta t}\} \approx \sum_{i=1}^{n_1} \left(\frac{\partial\{\delta_{t+\Delta t}\}}{\partial a_i}\Big|_{a=\overline{a}}\right)^2 \cdot \sigma_i^2$$
(35)

The partial derivative of $\ddot{\delta}_{t+\Delta t}$ with respect to a_i is given by

$$\frac{\partial\{\vec{\delta}_{t+\Delta t}\}}{\partial a_{i}} = b_{0} \left(\frac{\partial\{\delta_{t+\Delta t}\}}{\partial a_{i}} - \frac{\partial\{\delta_{t}\}}{\partial a_{i}} \right) - b_{2} \frac{\partial\{\vec{\delta}_{t}\}}{\partial a_{i}} - b_{3} \frac{\partial\{\vec{\delta}_{t}\}}{\partial a_{i}}$$
(36)

The partial derivative of $\dot{\delta}_{t+\Delta t}$ with respect to a_i is given by

$$\frac{\partial\{\dot{\delta}_{t+\Delta t}\}}{\partial a_{i}} = \frac{\partial\{\dot{\delta}_{t}\}}{\partial a_{i}} + b_{6}\frac{\partial\{\ddot{\delta}_{t}\}}{\partial a_{i}} + b_{7}\frac{\partial\{\ddot{\delta}_{t+\Delta t}\}}{\partial a_{i}}$$
(37)

The partial derivative of Eq.36 with respect to a_i is given by

$$\frac{\partial^2 \left\{ \ddot{\delta}_{t+\Delta t} \right\}}{\partial a_i^2} = b_0 \left(\frac{\partial^2 \left\{ \delta_{t+\Delta t} \right\}}{\partial a_i^2} - \frac{\partial^2 \left\{ \delta_t \right\}}{\partial a_i^2} \right) - b_2 \frac{\partial^2 \left\{ \dot{\delta}_t \right\}}{\partial a_i^2} - b_3 \frac{\partial^2 \left\{ \ddot{\delta}_t \right\}}{\partial a_i^2} \tag{38}$$

The partial derivative of Eq.37 with respect to a_i is given by

$$\frac{\partial^2 \left\{ \dot{\delta}_{t+\Delta t} \right\}}{\partial a_i^2} = \frac{\partial^2 \left\{ \dot{\delta}_t \right\}}{\partial a_i^2} + b_6 \frac{\partial^2 \left\{ \ddot{\delta}_t \right\}}{\partial a_i^2} + b_7 \frac{\partial^2 \left\{ \ddot{\delta}_{t+\Delta t} \right\}}{\partial a_i^2}$$
(39)

The mean value and variance of the displacement are obtained at time $t + i_1 \Delta t (i_1 = 2, 3, \dots, n_3)$ step-by-step.

The partial derivative of Eq.27 with respect to a_i is given by

$$\frac{\partial\{\sigma\}}{\partial a_i} = \frac{\partial[D]}{\partial a_i} [B] \{\delta_{t'}{}^d\} + [D] \frac{\partial[B]}{\partial a_i} \{\delta_{t'}{}^d\} + [D] [B] \frac{\partial\{\delta_{t'}{}^d\}}{\partial a_i}$$
(40)

The partial derivative of Eq.40 with respect to a_i is given by

$$\frac{\partial^{2} \{\sigma\}}{\partial a_{i}^{2}} = \frac{\partial^{2} [D]}{\partial a_{i}^{2}} [B] \{\delta_{t'}{}^{d}\} + 2 \frac{\partial [D]}{\partial a_{i}} \frac{\partial [B]}{\partial a_{i}} \{\delta_{t'}{}^{d}\} + 2 \frac{\partial [D]}{\partial a_{i}} [B] \frac{\partial \{\delta_{t'}{}^{d}\}}{\partial a_{i}} + [D] \frac{\partial^{2} [B]}{\partial a_{i}^{2}} \{\delta_{t'}{}^{d}\} + 2 [D] \frac{\partial [B]}{\partial a_{i}} \frac{\partial \{\delta_{t'}{}^{d}\}}{\partial a_{i}}$$

$$+[D][B]\frac{\partial^2 \left\{\delta_{t'}{}^d\right\}}{\partial a_i^2} \tag{41}$$

The stress is expanded at mean value point $\overline{a} = (\overline{a}_1, \overline{a}_2, \dots, \overline{a}_i, \dots, \overline{a}_{n_1})^T$ by means of a Taylor series. By taking the expectation operator for two sides of the above Eq.27, the mean of stress is obtained as

$$\mu\{\sigma\} \approx \{\sigma\}\Big|_{a=\overline{a}} + \frac{1}{2} \sum_{i=1}^{n_1} \frac{\partial^2\{\sigma\}}{\partial a_i^2}\Big|_{a=\overline{a}} \cdot \sigma_i^2$$
(42)

where, $\mu{\sigma}$ expresses the mean value of σ . The variance of σ is given by

$$Var\{\sigma\} \approx \sum_{i=1}^{n_1} \left(\frac{\partial\{\sigma\}}{\partial a_i}\Big|_{a=\overline{a}}\right)^2 \cdot \sigma_i^2$$
(43)

6. Numerical example

Figure 1 shows a four-bar linkage, or a crank and rocker mechanism. The establishment of differential equation system can be found in literature 10, 11, 12. The length of bar 1 is 0.075m, the length of bar 2 is 0.176m, the length of bar 3 is 0.29m, and the length of the bar 4 is0. 286m, the diameters of three bars are 0.02m. The torque T is 4Nm, the load F1 is 20sint N. The three bars are made of steel and they are regarded as three elements. Considering the boundary condition, there are 13 unit coordinates. Young's modulus is regarded as a random variable. For numerical calculation, the means of the Young's modulus within the three bars are $.2 \times 10^{11}$. N/m^2 and the variances of the Young's modulus are 10^{11} N^2/m^4 . Figure 2 shows the mean of the displacement at unit coordinate 11. Unit coordinate 11 is the deformation of the upper end of bar 3 in the vertical direction. The DSFEM simulates 1000 samples. The TSFEM produces an error of less than 0.5%. The CG produces an error of less than 0.1%. Figure 3 shows the variance of the displacement at unit coordinate 11. TSFEM produces an error of less than 1.0%. CG produces an error of less than 0.4%. Figure 4 shows the mean of stress at the top of bar 3. The TSFEM produces an error of less than 0.85%. The CG produces an error of less than 0.13%. Figure 5 shows the variance of stress at the top of bar 3. The TSFEM produces an error of less than 1%. The CG produces an error of less than 0.3%. The results obtained by the CG method and the TSFEM are very close to that obtained by the DSFEM. Table 1 indicates the comparison of CPU time when the mechanism has operated for six seconds.

Figure 6 shows a cantilever beam. The length, the width, the height, the Poisson's ratio, the Young's modulus and the load F are assumed to be random variables. Their means are 1m, 0.1m, 0.05m, 0.2, 2×10^{11} N/m², 100N. Their standard deviation are 0.2, 0.1, 0.1, 0.01, 10⁹, 0.1. Load subjected to the cantilever beam is Fsin(100t)N. It is divided into 400 rectangle elements that have 505 nodes. Figure 7 shows the mean of vertical displacement at node 505. DSFEM simulates 100 samples. The result obtained by the TSFEM produces an error of less than 2%. CG produces an error of less than 0.5%. Figure 8 shows the variance of vertical

230

displacement at node 505. The TSFEM produces an error of less than 3.0%. CG produces an error of less than 0.8%. Figure 9 shows the mean of horizontal stress at node 5. The TSFEM produces an error of less than 2.4%. CG produces an error of less than 0.9%. Figure 10 shows the variance of horizontal stress at node 5. The TSFEM produces an error of less than 3.2%. CG produces an error of less than 1.3%. Table 2 indicates the comparison of CPU time when the cantilever beam has operated for six seconds.

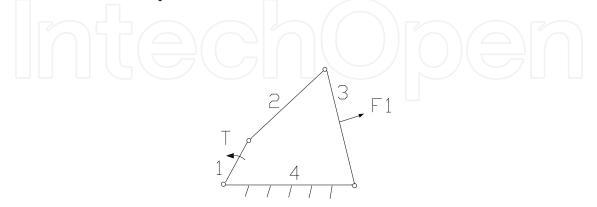


Fig. 1. A four-bar linkage

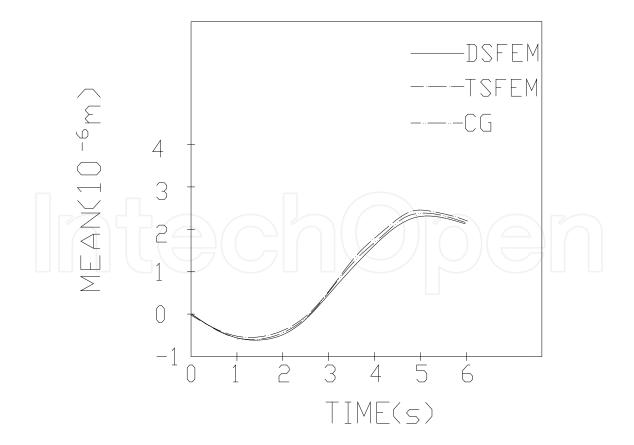


Fig. 2. The mean of displacement at unit coordinate 11 for $\sigma_E^2 = 10^{11}$

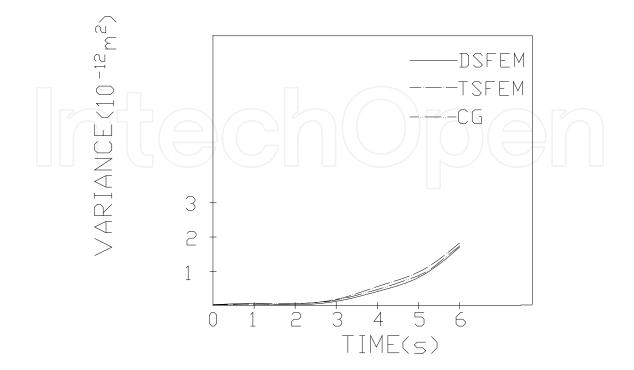


Fig. 3. The variance of displacement at unit coordinate 11 for $\sigma_E^2 = 10^{11}$

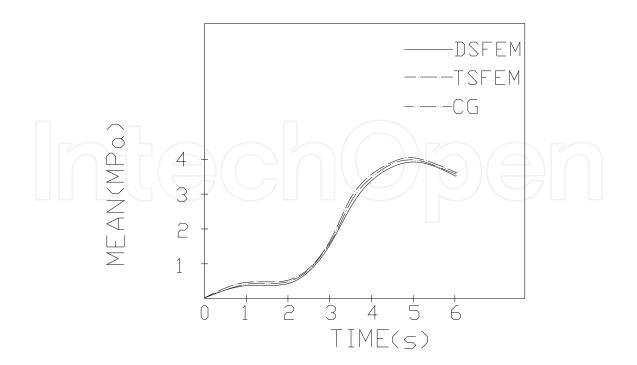


Fig. 4. The mean of stress at the top of bar 3 for $\sigma_E^2 = 10^{11}$

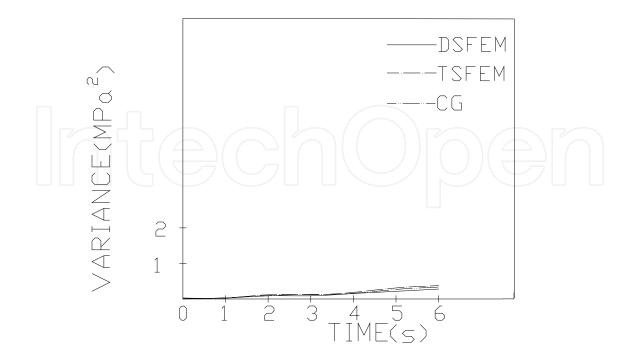


Fig. 5. The variance of stress at the top of bar 3 for $\sigma_E^2 = 10^{11}$

DSFEM	TSFEM	CG
19 seconds	4 seconds	14 seconds

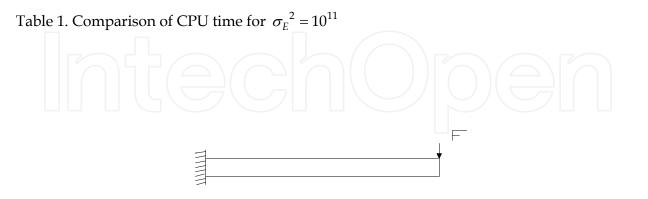


Fig. 6. A cantilever beam

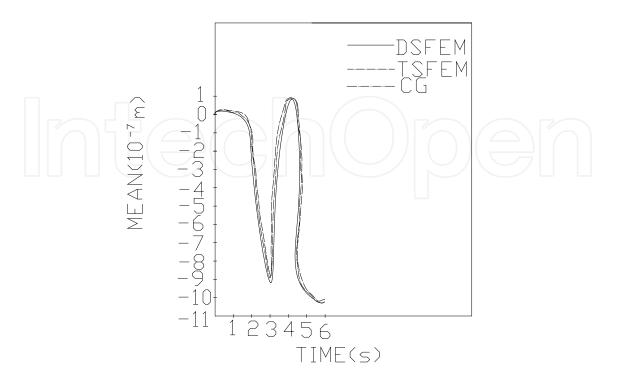


Fig. 7. The mean of vertical displacement at node 505

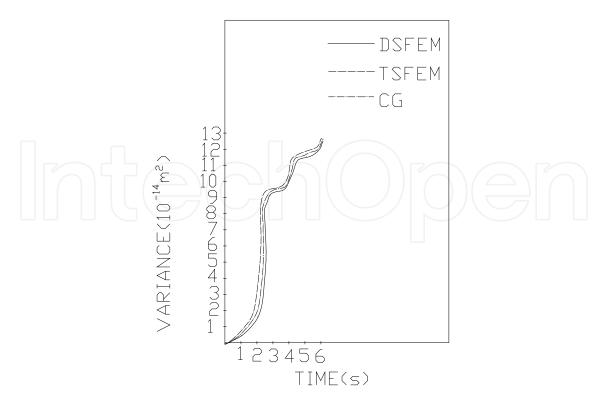


Fig. 8. The variance of vertical displacement at node 505

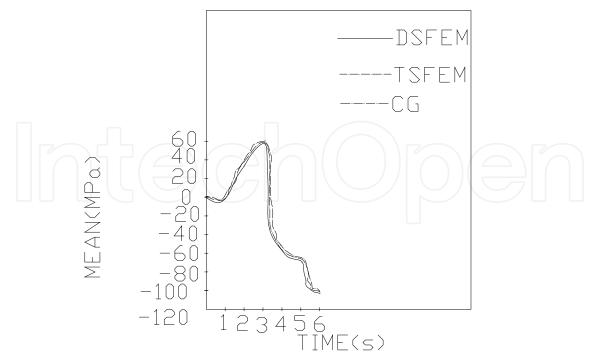


Fig. 9. The mean of horizontal stress at node 5

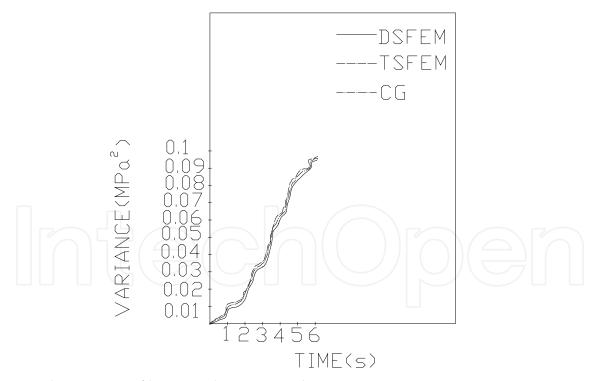


Fig. 10. The variance of horizontal stress at node 5

DSFEM	TSFEM	CG
3 hours 8 minutes 17 seconds	1 hour 45 minutes 10 seconds	40 minutes 24 seconds

Table 2. Comparison of CPU time

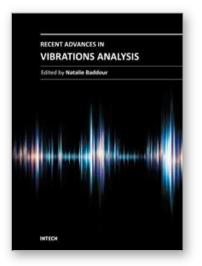
7. Conclusions

Considering the influence of random factors, the mechanical vibration in a linear system is presented by using the TSFEM. Different samples of random variables are simulated. The combination of CG method and Monte Carlo method makes it become an effective method for analyzing the vibration problem with the characteristics of high accuracy and quick convergence.

8. References

- J. Astill, C. J. Nosseir and M. Shinozuka. Impact loading on structures with random properties.J. Struct. Mech.,1(1972) 63-67
- [2] F. Yamazaki , M. Shinozuka and G. Dasgupta.. Neumann expansion for stochastic finite element analysis. J. Engng. Mech. ASCE. 114 (1988):1335-1354
- [3] M. Papadrakakis and V. Papadopoulos. Robust and efficient methods for stochastic finite element analysis using Monte Carlo Simulation. Comput. Methods Appl. Mech. Engrg. 134(1996)325 -340
- [4] G. B. Baecher and T. S. Ingra. Stochastic FEM in settlement predictions. J. Geotech. Engrg. Div.107 (1981)449-463.
- [5] K. Handa and K. Andersson. Application of finite element methods in the statistical analysis of structures. Proc. 3rd Int. Conf. Struct. Safety and Reliability, Trondheim, Norway (1981)409 -417.
- [6] T. Hisada and S. Nakagiri. Role of the stochastic finite elenent method in structural safety and reliability. Proc. 4th Int. Conf. Struct. Safety and Reliability ,Kobe, Japan(1985)385-394.
- [7] S. Mahadevan and S. Mehta. Dynamic reliability of large frames. Computers & Structures 47 (1993)57-67.
- [8] W. K. Liu, T. Belytschko and A. Mani. Probabilistic finite elements for nonlinear structural dynamics. Comput.Methods Appl. Mech. Engrg. 57(1986)61-81.
- [9] S.Chakraborty and S.S.Dey. A stochastic finite element dynamic analysis of structures with uncertain parameters. Int. J. Mech. Sci. 40 (1998)1071-1087.
- [10] A.G.,Erdman and G.N.,Sandor.A general method for kineto-elastodynamic analysis and synthesis of mechanisms.ASME, Journal of Engineering for Industry 94(1972) 1193-1205.
- [11] R.C.Winfrey.Elastic link mechanism dynamics.ASME, Journal of Engineering for Industry 93(1971)268-272.
- [12] D.A., Turcic and A.Midha.Generalized equations of Motion for the dynamic analysis of elastic mechanism system. ASME Journal of Dynamic Systems, Measurement ,and ,Control 106(1984)243-248.
- [13] M.Kaminski . Stochastic pertubation approach to engineering structure vibrations by the finite difference method . Journal of Sound and Vibration (2002)251(4), 651– 670.
- [14] Kaminski,M.On stochastic finite element method for linear elastostatics by the Taylor expansion..Structural and multidisciplinary optimization..35(2008),213-223.
- [15] Sachin K;Sachdeva;Prasanth B;Nair;Andy J;Keane.Comparative study of projection schemes for stochastic finite element analysis. Comput. Methods Appl. Mech. Engrg. 195(2006),2371-2392.
- [16] Marcin Kaminski.Generalized perturbation-based stochastic finite element in elastostatics. Computer & structures.85 (2007),586-594.

236



Recent Advances in Vibrations Analysis

Edited by Dr. Natalie Baddour

ISBN 978-953-307-696-6 Hard cover, 236 pages Publisher InTech Published online 09, September, 2011 Published in print edition September, 2011

This book covers recent advances in modern vibrations analysis, from analytical methods to applications of vibrations analysis to condition monitoring. Covered topics include stochastic finite element approaches, wave theories for distributed parameter systems, second other shear deformation theory and applications of phase space to the identifications of nonlinearities and transients. Chapters on novel condition monitoring approaches for reducers, transformers and low earth orbit satellites are included. Additionally, the book includes chapters on modelling and analysis of various complex mechanical systems such as eccentric building systems and the structural modelling of large container ships.

How to reference

In order to correctly reference this scholarly work, feel free to copy and paste the following:

Mo Wenhui (2011). Stochastic Finite Element Method in Mechanical Vibration, Recent Advances in Vibrations Analysis, Dr. Natalie Baddour (Ed.), ISBN: 978-953-307-696-6, InTech, Available from: http://www.intechopen.com/books/recent-advances-in-vibrations-analysis/stochastic-finite-element-method-inmechanical-vibration

INTECH

open science | open minds

InTech Europe

University Campus STeP Ri Slavka Krautzeka 83/A 51000 Rijeka, Croatia Phone: +385 (51) 770 447 Fax: +385 (51) 686 166 www.intechopen.com

InTech China

Unit 405, Office Block, Hotel Equatorial Shanghai No.65, Yan An Road (West), Shanghai, 200040, China 中国上海市延安西路65号上海国际贵都大饭店办公楼405单元 Phone: +86-21-62489820 Fax: +86-21-62489821 © 2011 The Author(s). Licensee IntechOpen. This chapter is distributed under the terms of the <u>Creative Commons Attribution-NonCommercial-ShareAlike-3.0 License</u>, which permits use, distribution and reproduction for non-commercial purposes, provided the original is properly cited and derivative works building on this content are distributed under the same license.



